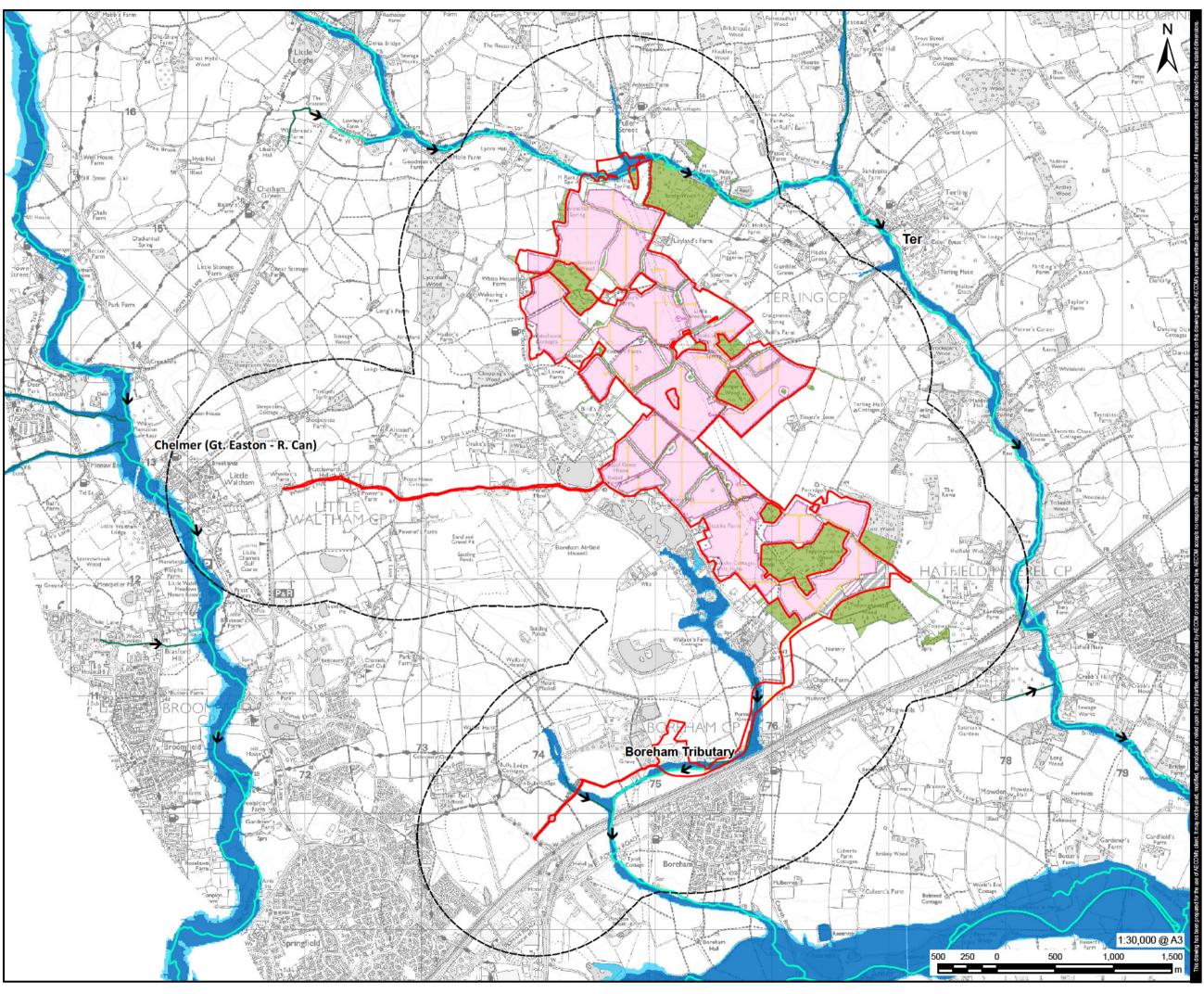


13. Annexes

Annex A - Development Parameter Plans





60640215 - LONGFIELD SOLAR FARM

CLIENT



CONSULTANT

AECOM Limited 3rd Floor Portwall Place, Portwall Lane, Bristol, BS1 6NA

LEGEND

LEGEND				
	Order Limits			
	1km Site Buffer			
→	Flow Direction			
	Main Rivers			
—	Ordinary Watercourse			
	Flood Zone 3			
	Flood Zone 2			
Indicat	tive Scheme Layout			
	Grid Connection Route			
	Hedgerow			
	Proposed Fencing			
	Primary Access			
	Secondary Access			
	PV Table			
	BESS Compound			
	Construction Compounds			
////.	Longfield Substation			
	Woodland			

NOTES

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ISSUE PURPOSE

Environmental Statement APFP Regulation: 5(2)(a)

PINS REFERENCE NUMBER

EN010118

FIGURE TITLE

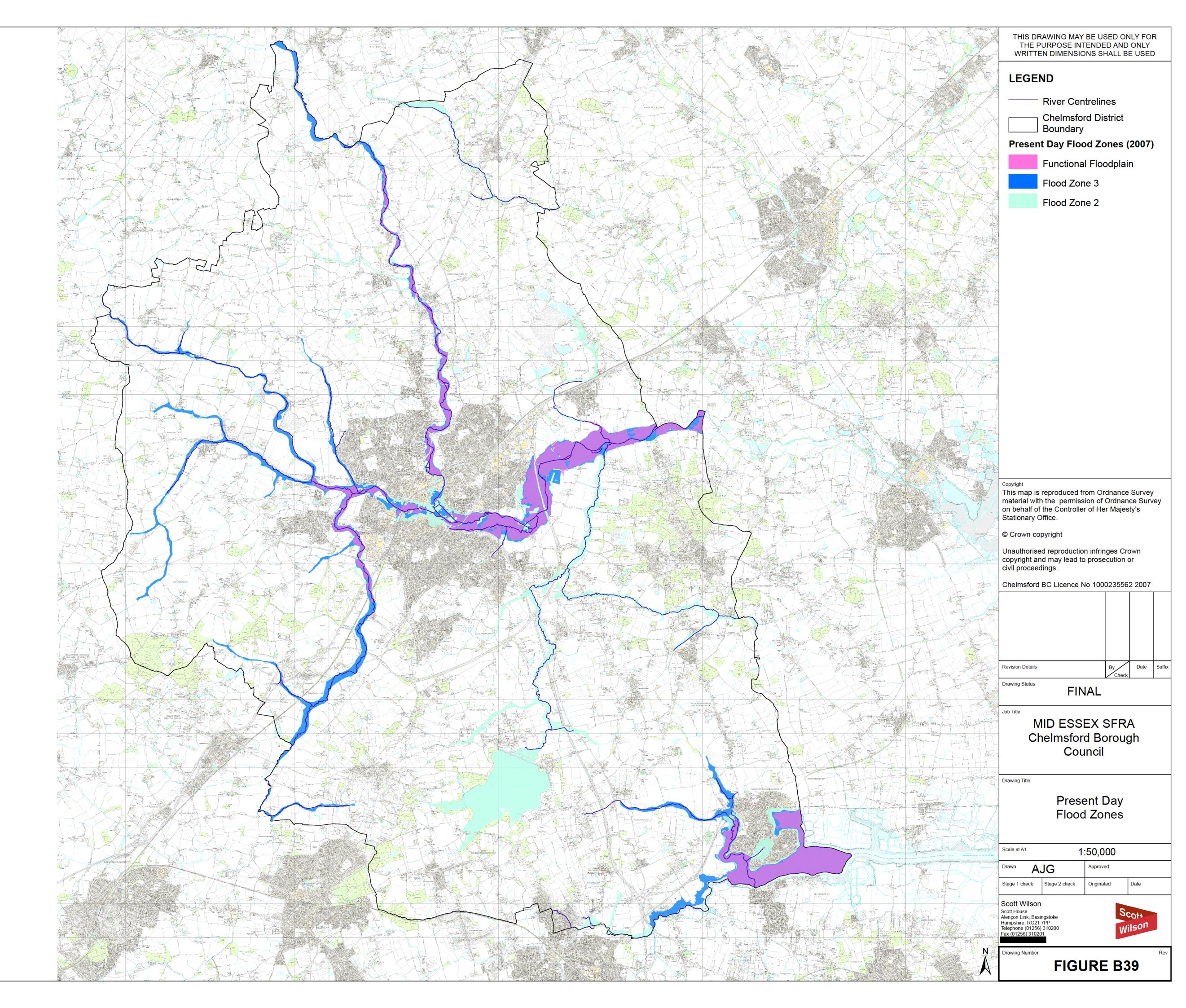
Fluvial Flood Zones including indicative concept design

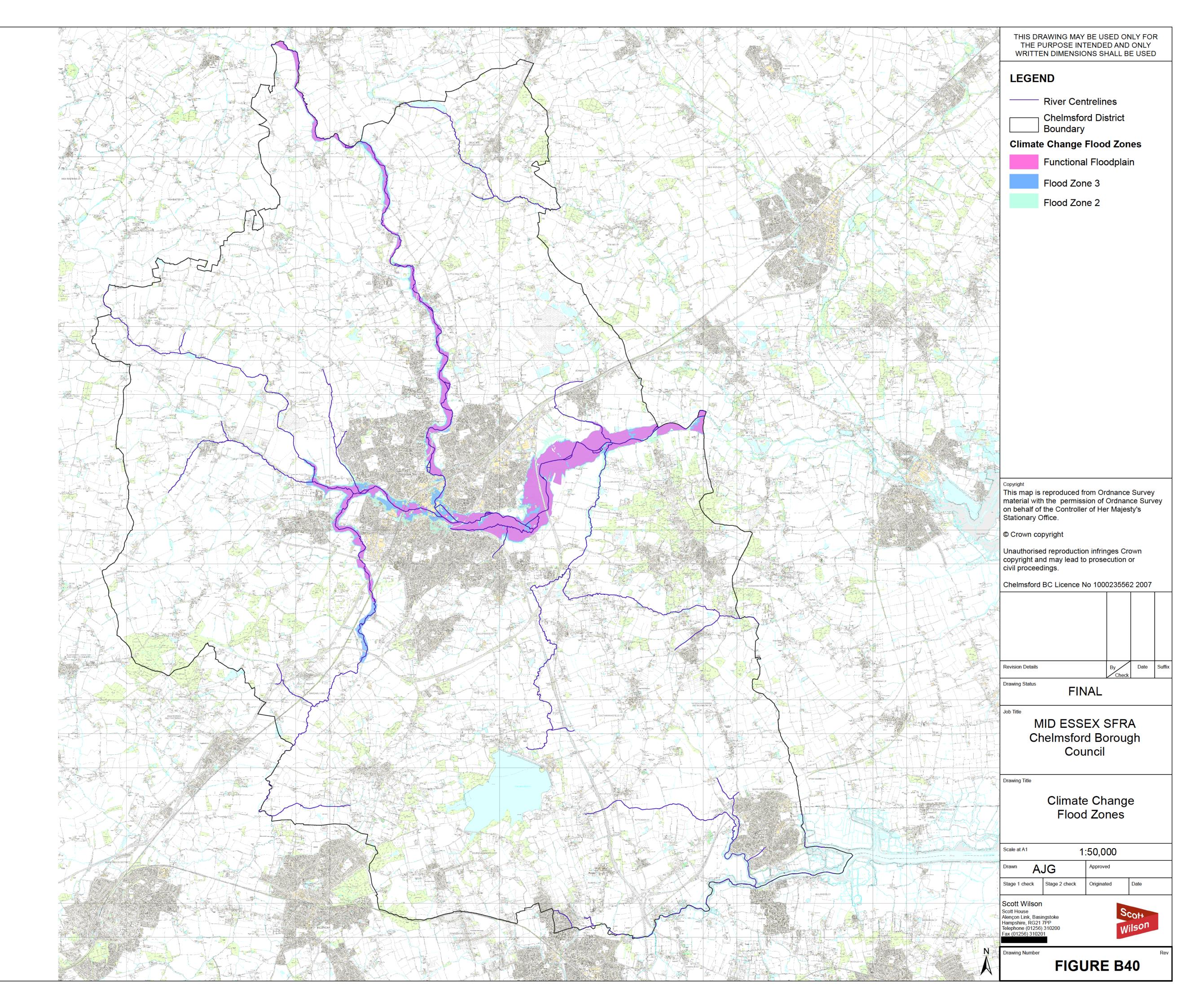
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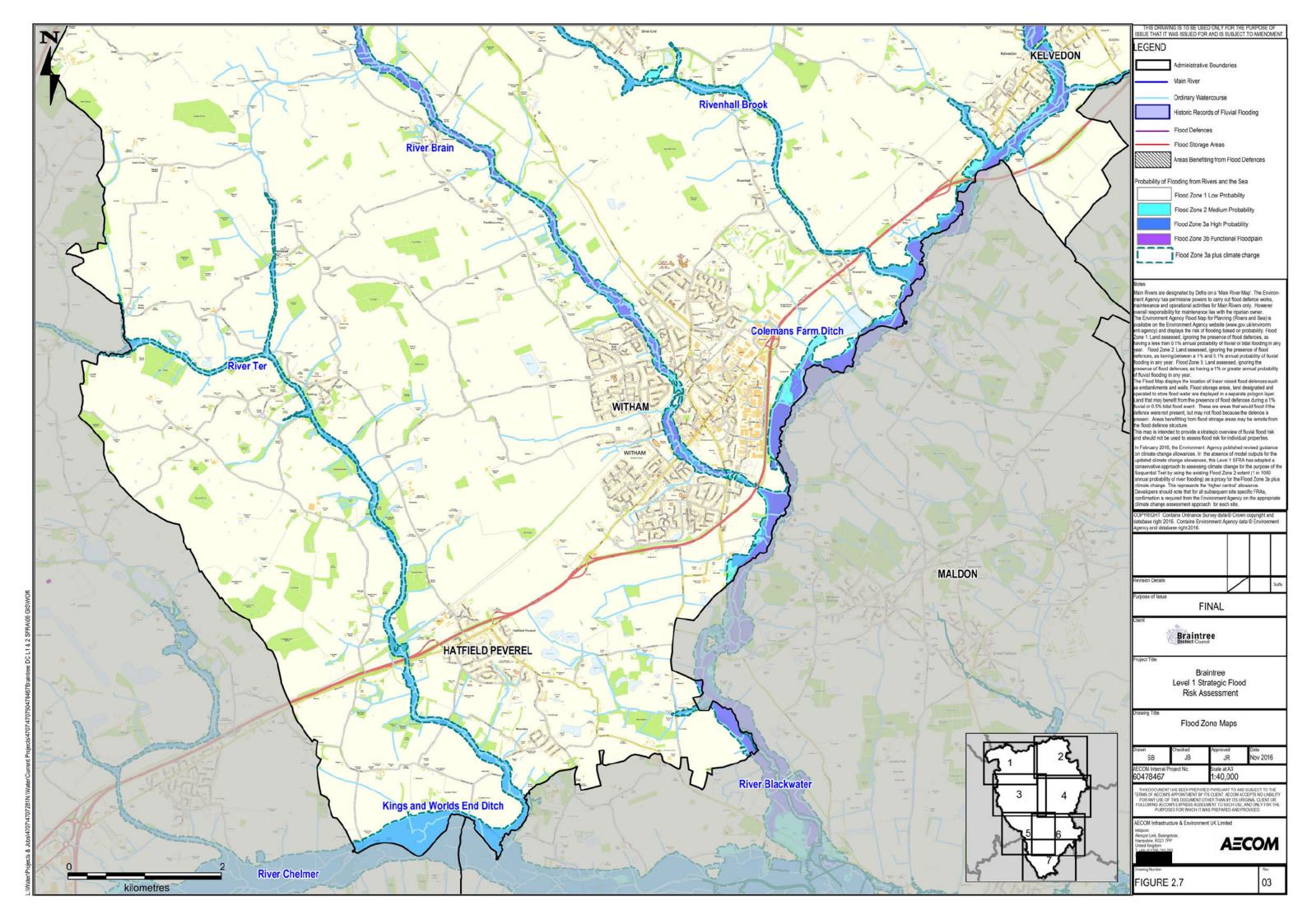
Figure 9-2b

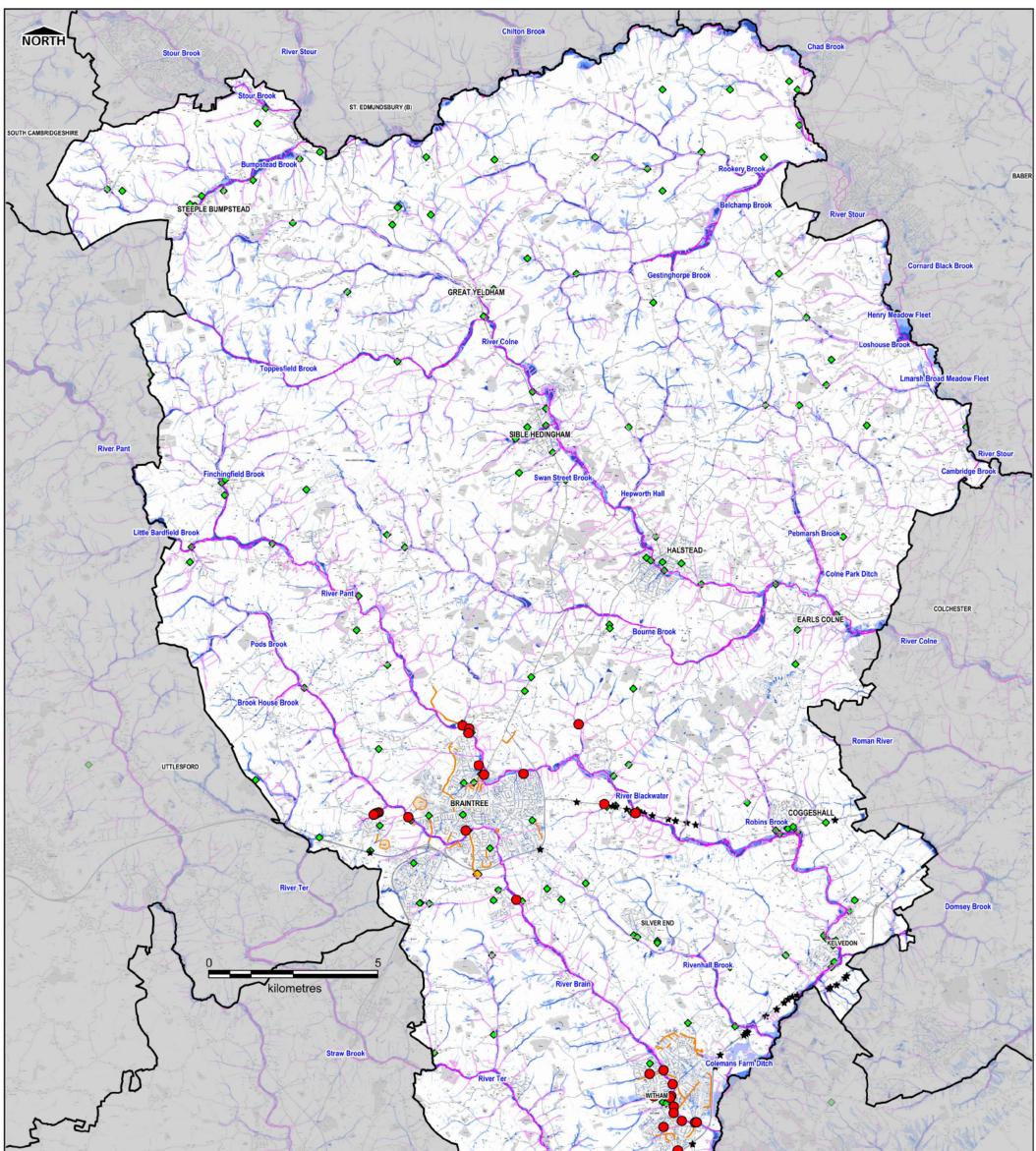


Annex B – Flood Risk Mapping

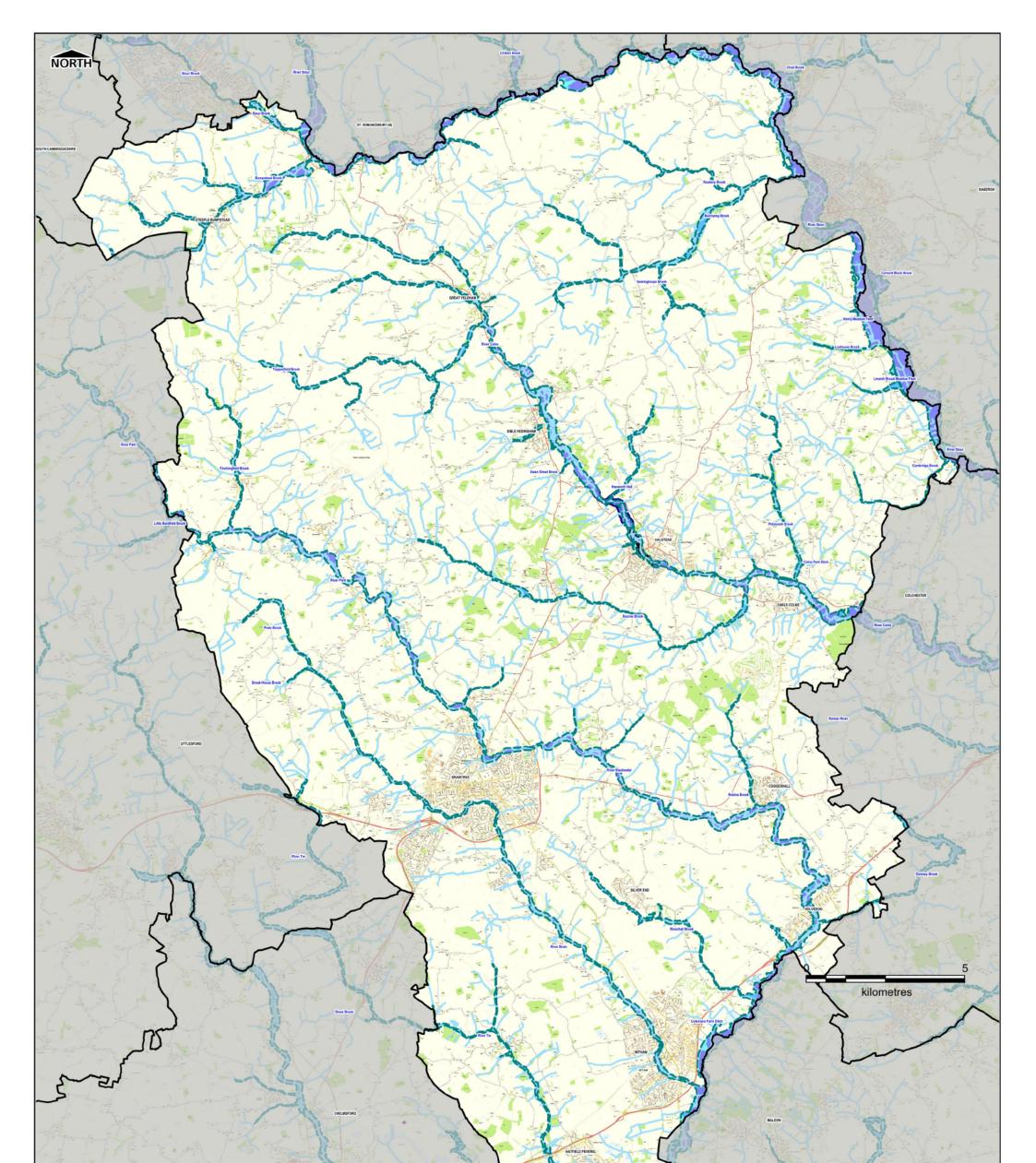


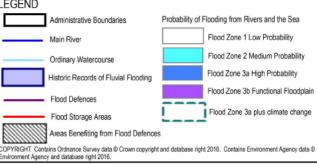






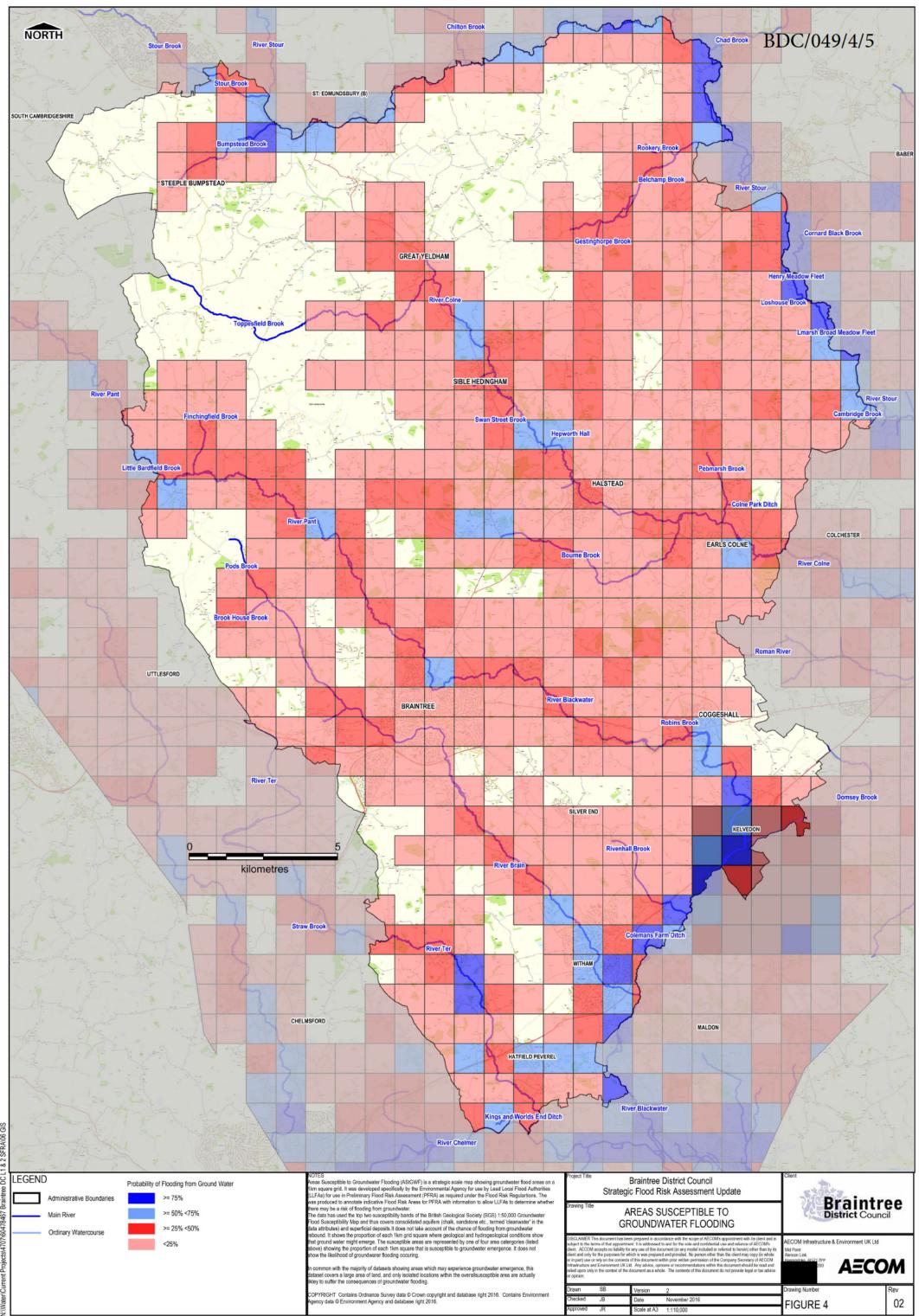
1 des		CHEL	MSFORD	MALDON	A.C
			HATFIELD PEVEREL Kings and Worlds End Ditch River. Cheimer	River Blackwater	
LEGENI		Probability of Flooding from Surface Water High (>3.3% AEP)	VOTES This map shows the predicted likelihood of surface water flooding based on the Environment Agency's Risk of Flooding from Surface Water (RoFSW) data, which may be subject to turther analysis in the future. Further information is provided on the Environment Agency webster livew gov ulderwinament agency).	Project Title Braintree District Council Strategic Flood Risk Assessment Update	Clent
	Administrative Boundaries Main River Ordinary Watercourse	Medium (1% - 3.3% AEP) Low (0.1% - 1% AEP)	The Bark from Surbace Water Roading is divided into categories. High cach year, the chance of floading is greater than 1 in 30(3.3%). Medium: each year, the chance of floading is greater than 1 in 100 (1%) and 1 in 30(3.3%). Low each year, the chance of floading is between 1 in 1000 (1%) and 1 in 100 (1%). Verture water year, if a chance of floading is less than 1 in 1000 (1%). The potential impact of surface water floading or avery according to the depth of the water, and its velocity (speed and direction that its flowing in). Surface water floading happens when rainstater does not dain away through the normal dramage systems or soak into the ground, but less on or flows over the ground instands. This spee floading can be difful to predice as its barries of to forceast exactly where or how much rain will bit in any stom.	Drawing Title Risk of Flooding from Surface Water	Braintree District Council
•	BDC Flood Summaries ECC Flood Incident Records Highways England Flood Records	Very Low (<0.1% AEP)	This map is intended to provide a strategic eventieve of surface water flood risk and should not be used to assesse flood risk for individual properties. Branthee District Courcel provide a strategic eventies for the lowns and villages of Brainhee as assessed by the local community, which is identified by the "BDC Flood Summaries". Essex County Courcel have provided information on surface water flooding from the 18th June 2015 fitroughout Biaintee and Witham.	DISCLAMER This document has been prepared in accordance with the scope of AECOM's appointment with its client and is subject to the terms of that appointment. It is addressed to and for the sole and confidential use and reliance of AECOM's dirent. AECOM accords to liability for any use of this document (or any model included or referred to herein) other that hysis dirent and only for the pupposes for which is was prepared and provided. It we preson other than the dirent may copy (in whole or in part) use or they not necessarily to its document within presons on the Company Secture you for AECOM intrastructure and Environment UK LM. Any advice, opinions or recommendations within the document within dired and and incleid upon only in the context of the document at an used. The contexts of this document dired hould be read and and control in the context of the document at an used.	AECOM Infrastructure & Environment UK Ltd Marpont Baeingsoble, HS217PP
ē	Known Flood Hotspots (EA & ECC)			Drawn SB Version 2 Checked JB Date November 2016 Approved JR Scale at A3 1:110.000	Drawing Number Rev FIGURE 3 02

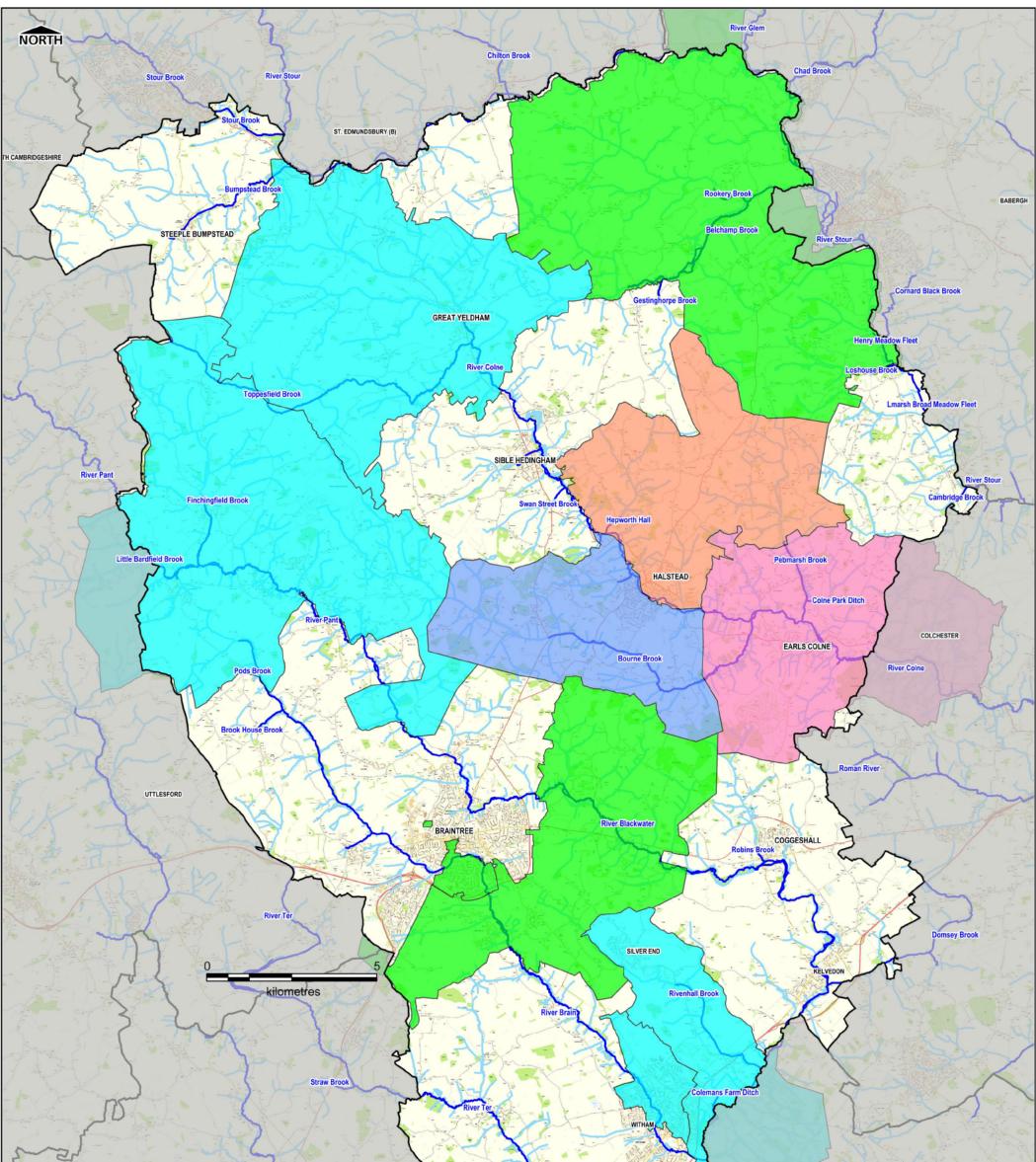




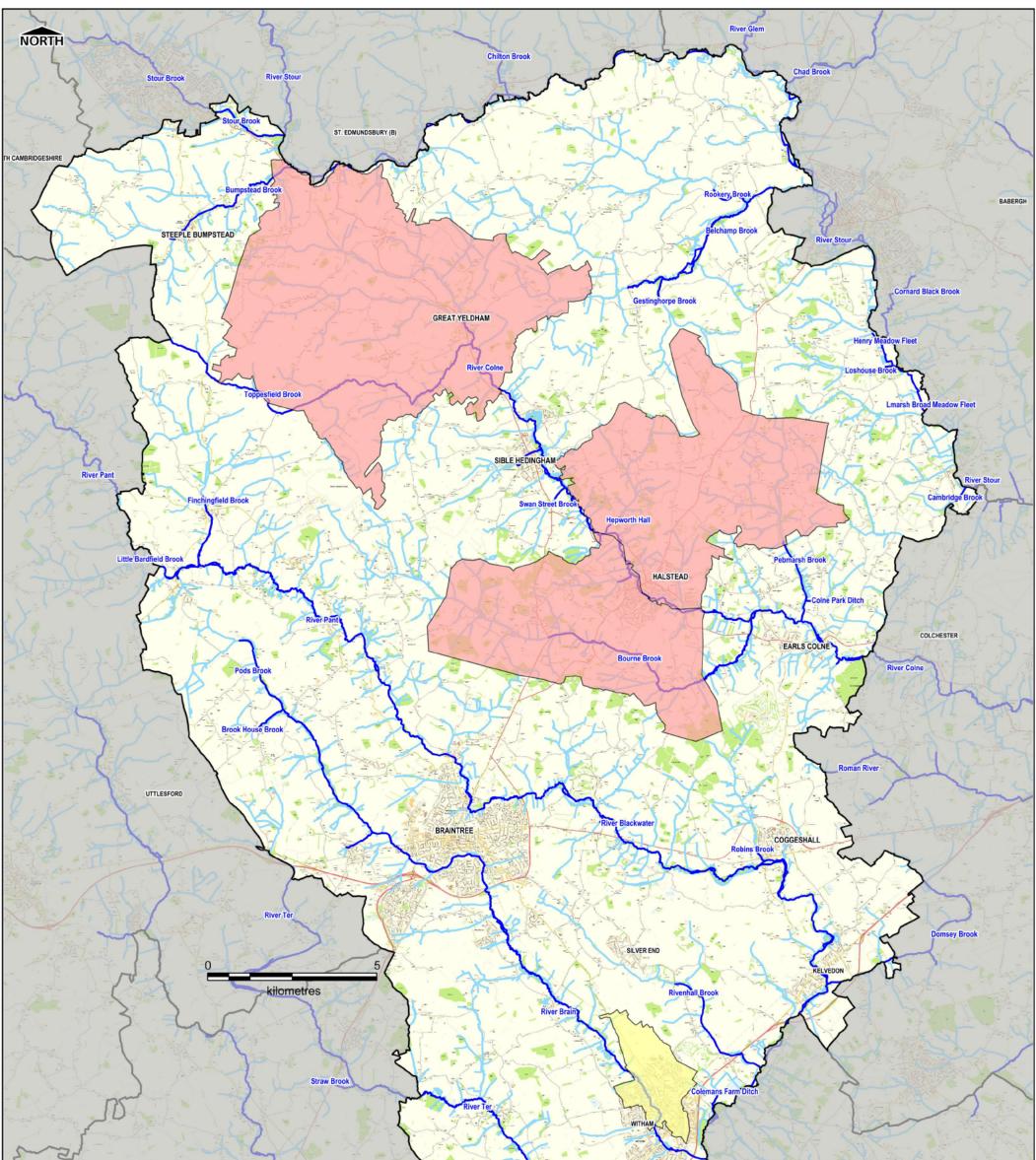
NOTES Main Rivers are designated by Defra on a 'Main River Map'. The Environment Agency has permissive powers to carry cut flood defence works, maintenance and operational activities for Main Rivers only. However overall responsibility for maintenance lies with the rightan owner. An Ortifiany Waterocuve is a waterocuve that does not form part of a Main River. This includes all rivers and streams and all ditches, drains, cuts, culverts, dikes, sluces (other than public severs within the meaning of the Viater Industry Act 1991) and passages, through which water flows according to the Land Drainage Act 1991. The Environment Agency Flood Map for Planning (River and Sea) is available on the Environment Agency Weblite (www.gov.uk/anvironment-agency) and displays the risk of flooding based on probability. Flood Zons 1: Land assessed, granting a the resence of flood defences, as having a less than 0.1% annual probability of fluvial of todal flooding in any year. Flood Zons 2: Land assessed, ingring the presence of flood defences, as having a less than 0.1% annual probability of fluvial flooding in any year. Flood Zons 2: Land assessed, ingring the presence of flood defences, as having a leves and 10.1% annual probability of fluvial flooding in any year. Flood Zons 2: Land assessed, ingring the presence of flood defences, as having a leves and 10.1% annual probability of fluvial flooding in any year. Flood Zons 2: Land assessed, ingring the presence of flood defences as a having a live of the defence as presence as the advertised of the defence were not present, but may not flood because the defence is present. This map is intended to provide a strategic overview of fluvial flooding is any and the origin defence as the would flood if the defence were not present, but may not flood because the defence is present. These are areas thet would flood if the defence were not present, but may not flood because the defence is present. These are areas thet would flood if the defence were not present, but may not flood because the

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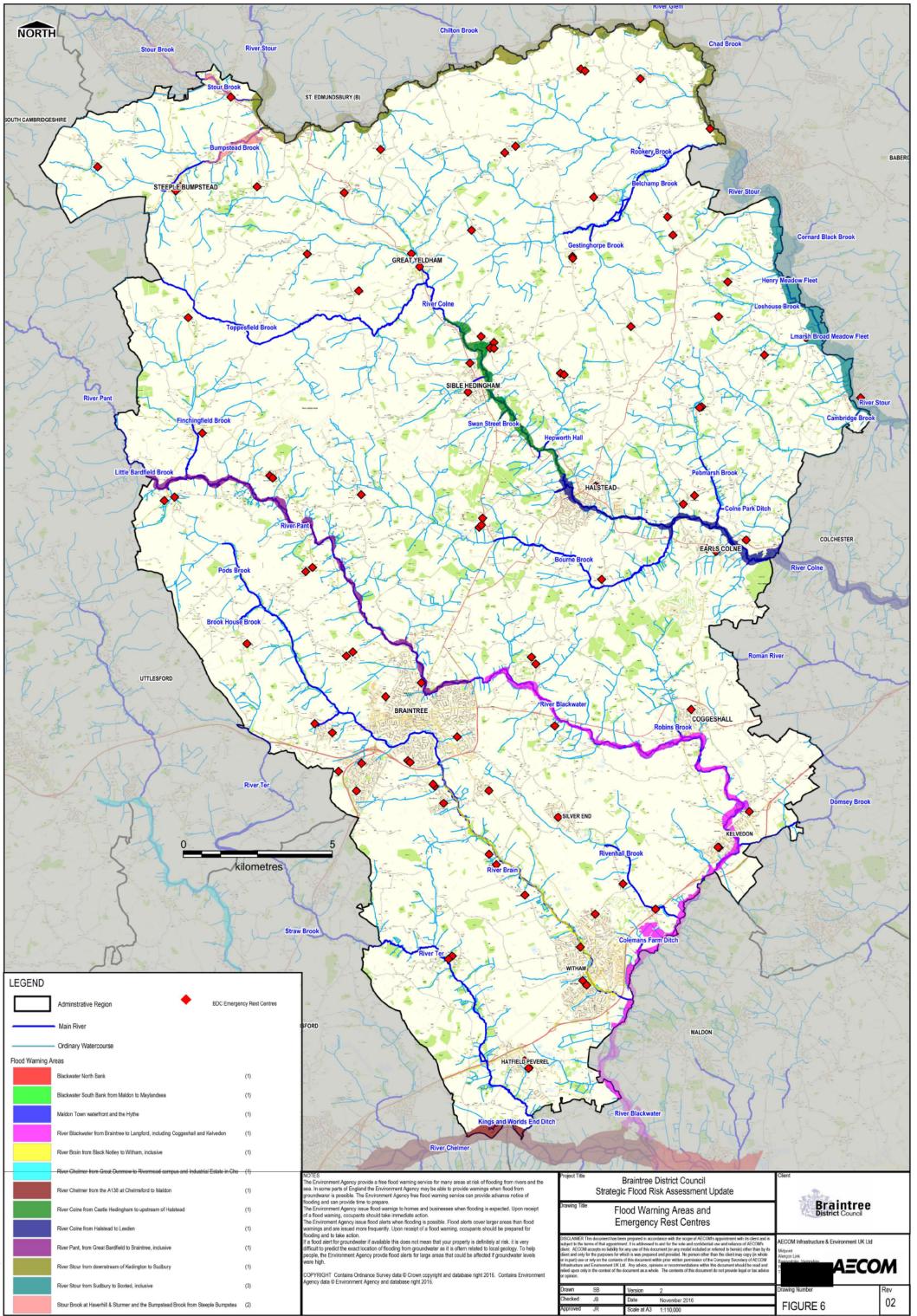


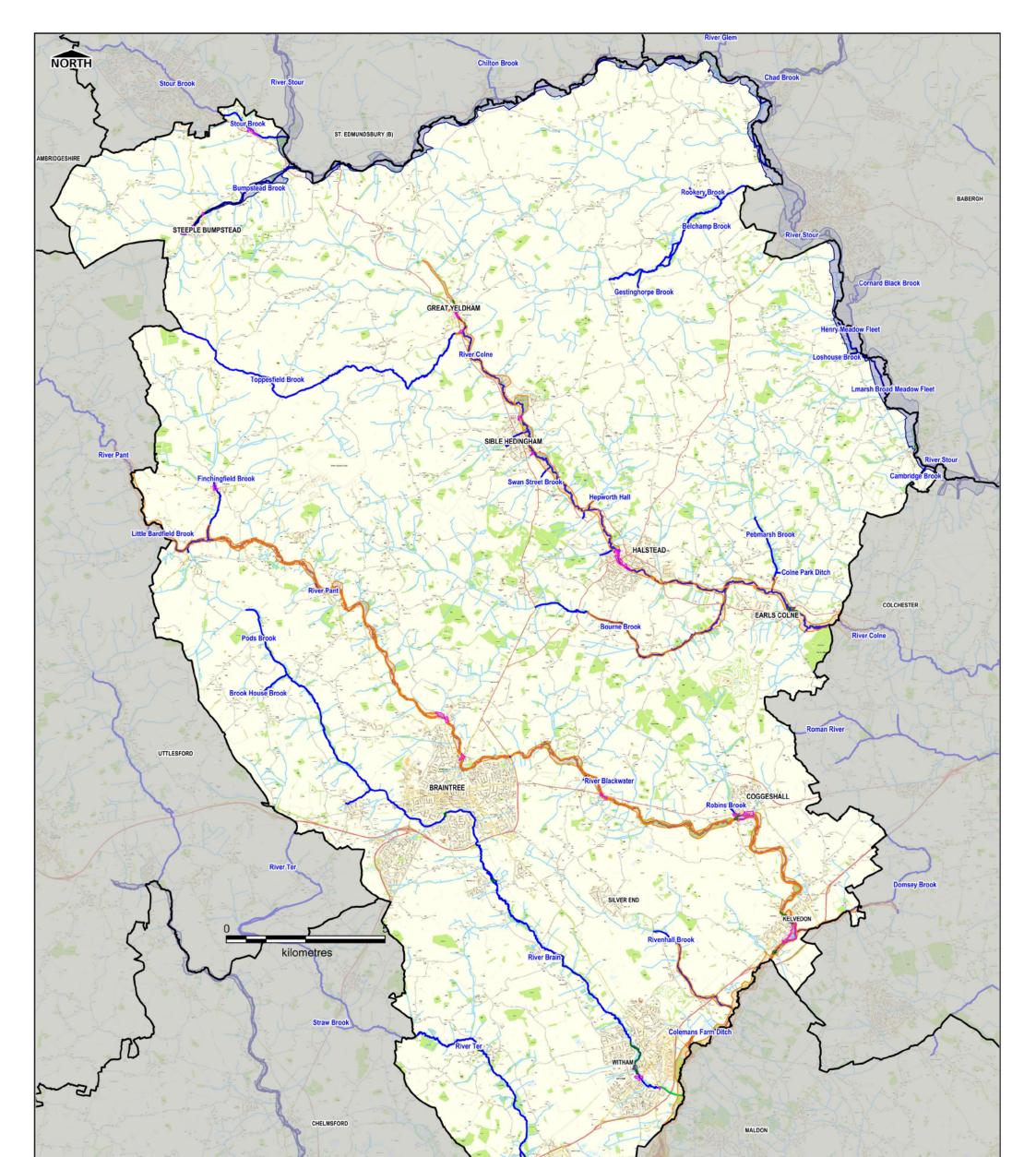


CHELMSF	ORD HATFIELD PEVEREL Kings and Worlds End Ditch River Cheimer	MALDON River Blackwater	
EXternal Sewer Flood Incidents	NOTES Anglian Water Services has provided an extract from their DGS Flood Register for the study area. Due to data protection requirements the data has not been provided at individual property level, rather the register comprises	Project Title Braintree District Council	Clent
Administrative Boundaries	which the last 10 years.	Strategic Flood Risk Assessment Update	Braintroo
Main River Ordinary Watercourse	t should be noted that records only appear on the DG5 register where they have been reported to AWS, and as such they may not include all instances of sever flooding.	External Sewer Flood Incidents	Braintree District Council
3	Furthermore given that AWS target these areas for maintenance and improvements, areas that experienced looding in the past may not longer be at greatest risk of flooing in the future.	DISCLAIMER. This document has been prepared in accordance with the scope of AECOMs appointment with its client and is subject to the terms of that appointment. It is addressed to and for the sole and coefidential use and reliance of AECOM's	AECOM Infrastructure & Environment UK Ltd
5	This map is intended to provide a strategic overview of sewer flood risk and should not be used to assess flood risk individal proceeties.	client. AECOM accepts to liability for any use of this document (or any model included or referred to herein) other than by the direct and only for the purposes for which is was prepared and provided. No presen other than the direct many coay (in whole or in part) use or rely on the contents of this document within prior written permission of the Company Secretary of AECOM	Midpoint Alençon Link Basingstoke, Hampshire,
6 ett	COPYRGHT Contains Ordnance Survey data @ Crown copyright and database right 2016. Contains Environment Agency data @ Environment Agency and database right 2016.	infradructure and Environment UK LM. Any advice, coincins or recommendations within this document should be read and instell upon only in the context of the document as a whole. The contents of this document do not provide legal or tax advice or opinion.	
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CHELMSFC	DRD HATFIELD PEVEREL Kings and Worlds End Ditch River Chelmer	MALDON River Blackwater	
Administrative Boundaries	NOTES Anglian Water Services has provided an extract from their DGS Flood Register for the study area. Due to data protection requirements the data has not been provided at individual property level; rather the register comprises the number of properties within 4 digit postcode areas that have experienced flooding either internally or externally	Project Title Braintree District Council Strategic Flood Risk Assessment Update	Client
Main River Ordinary Watercourse	It should be noted that records only appear on the DG5 register where they have been reported to AWS, and as such they may not include all instances of sewer flooding.	Drawing Title Internal Sewer Flood Incidents	Braintree District Council
Projects4707	Eurthermore given that AWS target these areas for maintenance and improvements, areas that experienced looding in the past may not longer be at greatest risk of flooing in the future. This map is intended to provide a strategic overview of sewer flood risk and should not be used to assess flood risk individal properties. COPYRIGHT Contains Ordnance Survey data © Crown copyright and database right 2016. Contains Environment Agency data © Environment Agency and database right 2016.	DISCLAMER This documenthas been prepared in accordance with the scope of AECOM's appointment with its client and is subject to the terms of that appointment. It is addressed to and for the sole and colidarial use and relations of AECOM's dent. AECOM accords no lisibility from use of the document within (to any modif included or relent of herein) there they tight dent and only for the purposes for which is use prepared and provider. It has presended in the first the terms (copy) (in whole in sparity use or region the contents of the document within the relations of the Company Section VI. Lth. Any advice, opinions or recommendations within the document within the index provides and the model of the contents of the sole of the contents of this document is hold be read and index provide using the context of the document at an whole. The contents of this document document and the or opinion.	AECOM Infrastructure & Environment UK Ltd Matoon Link Besingstok, Hampshire, Egg1 IPPLUK
Purpose FINAL		Drawn SB Version 2 Checked JB Date November 2016 Approved JR Scale at A3 1:110,000	FIGURE 5.2 02





LI & 2 SERRANDE GIS		HATFIELD PEVEREL Kings and Worlds End Ditch River Chelmer	River Blackwater
Administrative Boundaries		NOTES Wain Rivers are designated by Defra on a 'Main River Map'. The Environment Agency has permissive powers to carry out flood defence works, maintenance and operational activities for Main Rivers only. However overall responsibility for maintenance its with the riparian owner.	Project Title Braintree District Council Strategic Flood Risk Assessment Update
Main River Ordinary Watercourse	Historic Flood Outlines Historic Flood Record	An Ordinary Watercourse is a watercourse that does not form part of a Main River. This includes all rivers and streams and all ditches, drains, cuts, culverts, dives, sluices (other than public severs within the meaning of the Water Industry Act 1991) and passages, through which water flows according to the Land Drainage Act 1991.	Drawing Title Environment Agency Historic Flood Outlines
ojects/4707/4	1947 Flood Outline 1970 Flood Outline	COPYRIGHT Contains Ordnance Survey data © Crown copyright and database right 2016. Contains Environment Agency data © Environment Agency and database right 2016.	DISCLAMER This document has been prepared in accordance with the scope of AECOMs appointment with its clent and is ubject to the terms of that appointment. It is addressed to and for the sela and ordifactial use and reliance of AECOMs dater and only for the purposes for which is was prepared and provided. No person other Change in other cleanses (in which in party use or relianse) in the document within the clean may could be reliand to be the score of the Company's in party use or relianse of the Company's cleanses of the Company's cleanses of the Company's cleanses of the company's the cleanses of the document within the document shead be read and eliest upon only in the contect of the contents of the Company's cleanses of the Company's Company's cleanses of the Company's the cleanses of the Company's cleanses of the Company's Company's cleanses of the Company's the cleanses of the Company's cleanses of the Company's Company's cleanses of the Company's the cleanses of the Company's cleanses of the Company's Company's cleanses of the Company's the cleanses of the Company's cleanses of the Company's Company's cleanses of the Company's cleanses of the Company's cleanses of the Company's Company's cleanses of the Company's cleanses of the Company's Company's cleanses of the Company's cleanses of the Company's cleanses of the Company's Company's cleanses of the Company's cleanses of the Company's cleanses of the Company's Company's cleanses of the Company's cleanses of the Company's cleanses of the Company's Company's cleanses of the Company's cleanses of the Company's cleanses of the Company's Company's cleanses of the Compan
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N:Water(C	2009 Flood Outline	Purpose FINAL	Checked JB Date November 2016 Approved JR Scale at A3 1:110,000 FIGURE 7



Annex C – Additional Mapping



Flood map for planning

Your reference Longfield Location (easting/northing) 575399/213026

Created 16 Mar 2021 9:19

Your selected location is in flood zone 3, an area with a high probability of flooding.

This means:

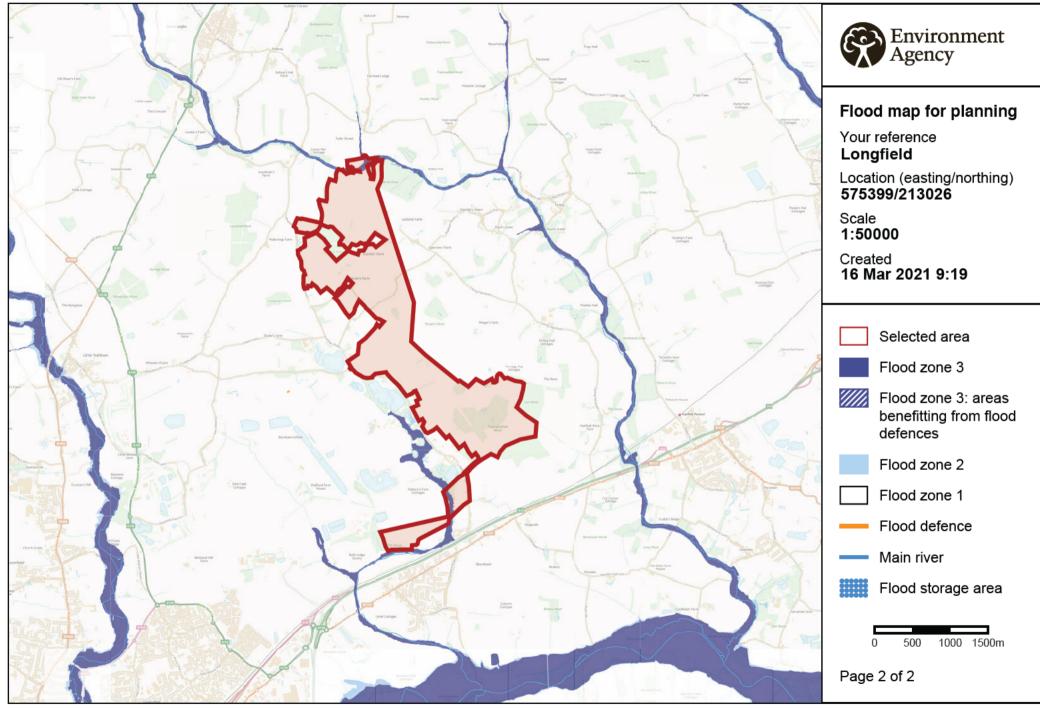
- you must complete a flood risk assessment for development in this area
- you should follow the Environment Agency's standing advice for carrying out a flood risk assessment (see www.gov.uk/guidance/flood-risk-assessment-standing-advice)

Notes

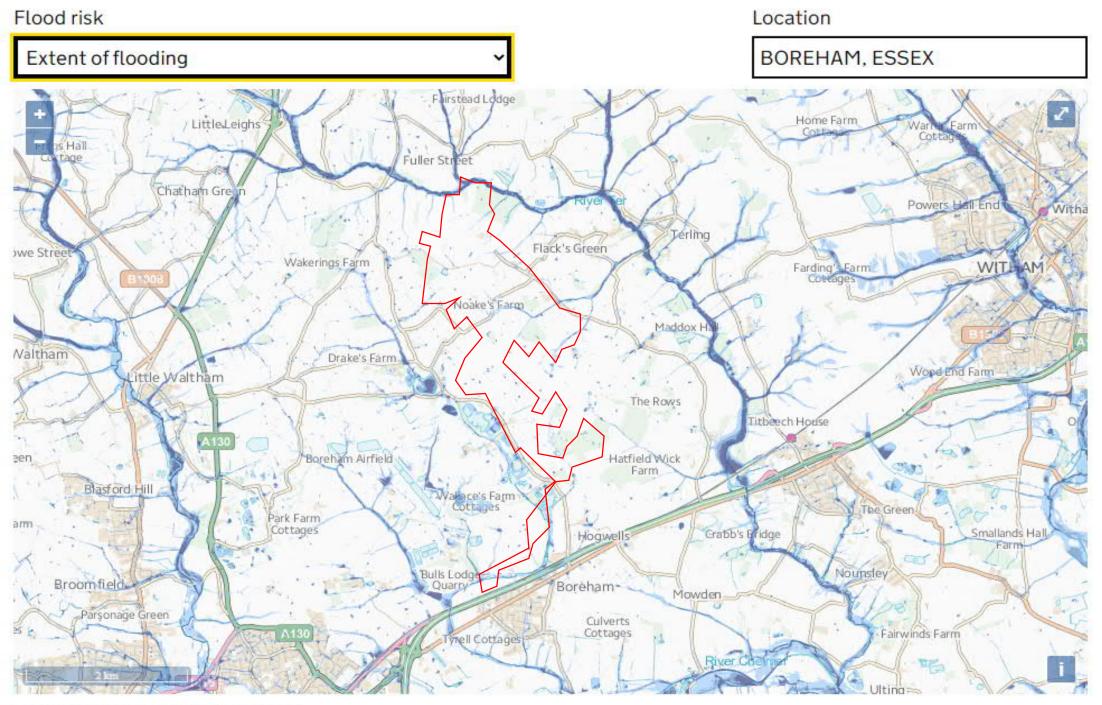
The flood map for planning shows river and sea flooding data only. It doesn't include other sources of flooding. It is for use in development planning and flood risk assessments.

This information relates to the selected location and is not specific to any property within it. The map is updated regularly and is correct at the time of printing.

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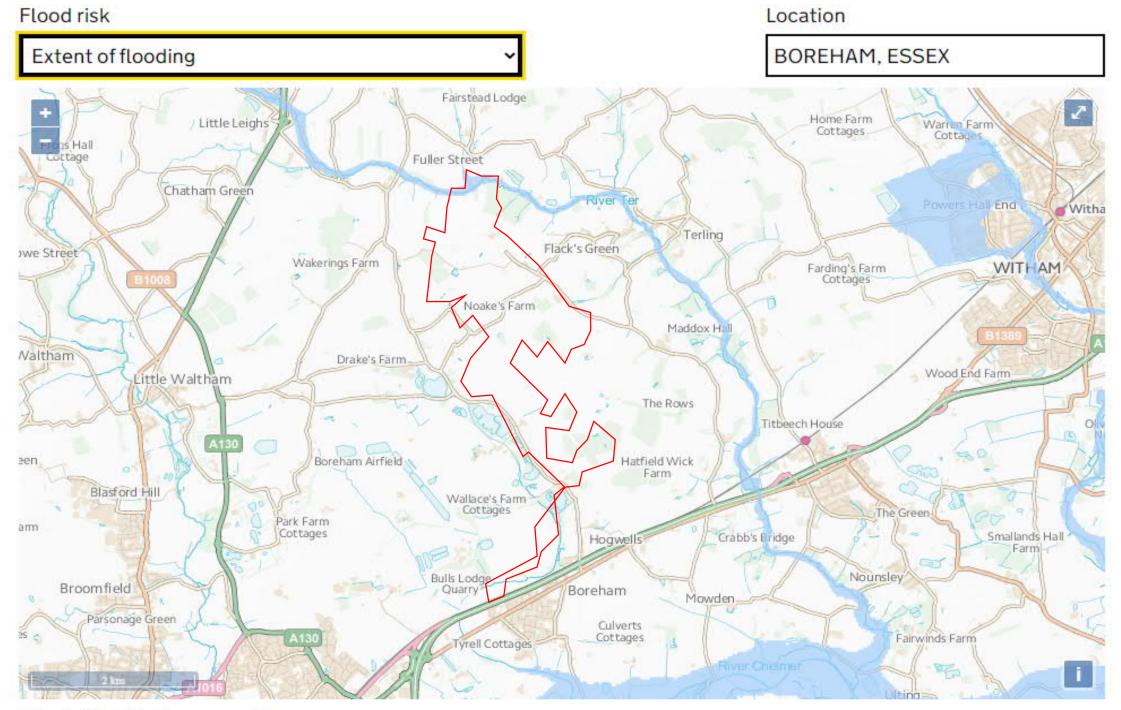


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Extent of flooding from surface water

High Medium Low Very low Cocation you selected



Extent of flooding from reservoirs



Annex D - Pluvial Modelling Report



SURFACE WATER MODELLING TECHNICAL NOTE

LONGFIELD SOLAR FARM

LONGFIELD SOLAR ENERGY FARM LTD

FEBRUARY 2022







1 INTRODUCTION

1.1 Project Background

Arcus Consultancy Services Ltd (Arcus) has been instructed by Longfield Solar Energy Farm Ltd to produce a surface water model at the location of the proposed Longfield Solar Farm (the Development) located north of Boreham village, Chelmsford at National Grid Reference E 576665, N 212070 (the DCO Site).

The purpose of the surface water modelling is to confirm the existing surface water depths and flow routes at the Site and surrounding land in order to incorporate surface water betterment measures as part of the biodiversity measures proposed as part of the Development.

2 METHODOLOGY

2.1 Initial Design Parameters

The surface water characteristics of the Site has been modelled in a 2D simulation utilising Flood Modeller 5.0 software and Alternating Direction Implicit (ADI) solver.

The topography at the Site and surrounding catchment is represented in the model by 2019 LiDAR data to 1 metre (m) resolution (TL71 and TL70).

All model runs are set to a timestep of 2 seconds and grid sizing of 4 m. To enable the model to run at such a grid size the Site was split into a northern and southern domain, as shown in Plate 1, to incorporate the Site and surrounding catchment.

The northern and southern domains are approximately 5.7 square kilometres (km^2) and 5.2 km^2 in area respectively.

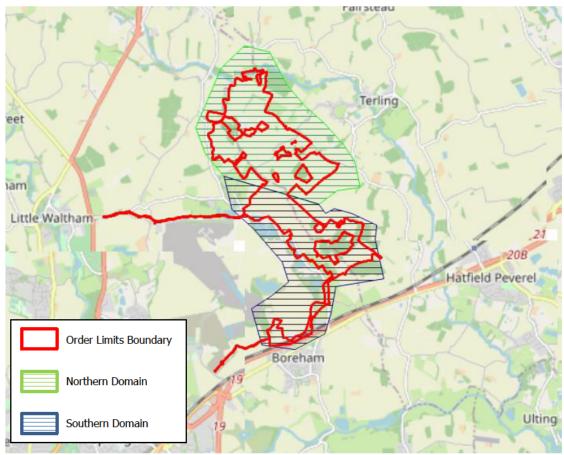


Plate 1 – 2D Model Domain (Taken from Flood Modeller)

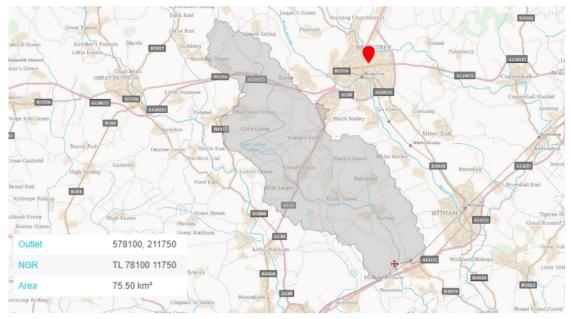
2.2 Hydrology Data

To develop hyetographs, catchment descriptors have been imported from the UK Centre for Ecology and Hydrology (CEH) Flood Estimation Handbook (FEH) web map¹ for a number of return periods as a 100 % rural model from a catchment of 75.5 km² as shown in Plate 2.

¹ UK Centre for Ecology and Hydrology, Flood Estimation Handbook. [Online]. Available at:



Plate 2 – FEH Catchment



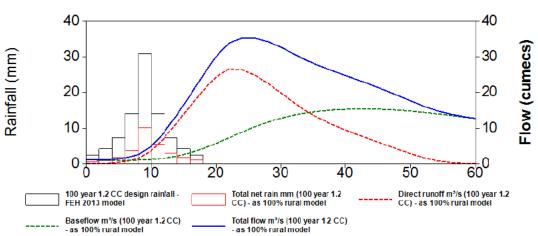
To account for the percolation capacity of the catchment the net loss rainfall data has been utilised within the Revitalised Flood Estimation Handbook 2 (ReFH2) software to generate a hyetograph which represents rainfall loses within the catchment. The summer profile shows the greatest rainfall intensities and has been utilised to demonstrate a 'worst-case scenario'.

The 1:100-year return period is the appropriate return period for the Development and has been utilised.

The Development has an operational lifetime of less than 50 years. In accordance with Environment Agency (EA) climate change allowances² a 20 % uplift has been applied to the 1:100-year hyetograph to account for increases in rainfall intensities associated with climate change up to 2069 with a hyetograph shown in Plate 3.

Outputs from the REFH2 hyetograph are shown in Appendix A.

Plate 3 – 1 in 100-Year (+20%) REFH2 Hyetograph



100 year 1.2 CC - as rural

² Environment Agency, Flood Risk Assessment: Climate Change Allowances (2021). [Online]. Available at: <u>https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances</u>



2.3 Iterative Design

To measure the potential impact of the Development on surface water flood depths and flows the modelling process has been conducted through an iterative process. This process has been conducted in three key modelling phases; the baseline scenario, the refined baseline scenario and the operational phase scenario. The design details of each phase are detailed in the following sections.

2.3.1 Baseline Scenario

The baseline scenario model has been developed to identify the existing surface water flow characteristics at the Site and any areas of potential surface water flood risk at or emanating from the Site.

Ordnance Survey (OS) buildings data has been incorporated into the baseline scenario model with existing buildings within the model domains raised above topography.

OS roads data has been incorporated into the baseline scenario with existing roads within the model domains depressed 0.1 m below existing topography.

A universal Manning's N roughness value of 0.03 has been incorporated into the model to represent short grass pasture grounds in accordance with Chow 1959³.

2.3.2 Refined Baseline Scenario

The refined baseline scenario has been developed to add further characteristics of the catchment into the baseline scenario to identify the surrounding surface water characteristics.

Consultations with surrounding landowners were conducted in September 2021 by Pershing Consultants where the baseline scenario methodology and outputs were reviewed. Consultations identified the presence of a 1 m high embankment located to the east of Stocks Farm which diverted surface water flow routes away from the property and within the Site. The embankments have therefore been incorporated into the model and raised 1 m above existing topography. The location of the embankment is show in Plate 3.

³ Chow, Manning's N Values for Channels, closed Conduits Flow Partially Full and Corrugated Metal Pipes (1959). [Online]. Available at:





Aerial imagery and photographs obtained from an Arcus hydrology site walkover in July 2021 have been assessed to identify existing ground conditions and land use at the Site and surrounding areas. The Manning's N roughness value has been amended from the universal value for any areas identified as woodland, roads and tracks as detailed in Plate 4 and Table 1 with values derived from Chow 1959.

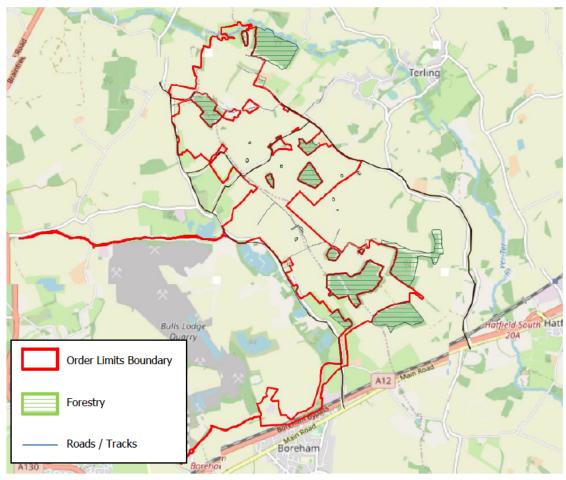


Plate 4 – Manning's N Roughness Values (Taken from Flood Modeller)

Table 1: Manning's N Roughness Values

Ground Condition Type	Manning's N Value Applied	Chow 1959 Definition
Woodland	0.15	Trees – dense willows, summer, straight
Tracks	0.025	Earth – no vegetation
Roads	0.02	Concrete – on good excavated rock

Aerial imagery and photographs have been assessed to identify existing roads and tracks not incorporated within the OS roads data. These have been depressed by 0.1 m below existing topography.

2.3.3 Operational Phase Scenario

The operational phase scenario has been developed to identify the potential surface water impact of infrastructure associated with the Development and to quantify the potential betterment of mitigation measures proposed.

The existing flow routes and surface water flood depths have been assessed form the refined baseline scenario to confirm proposed betterment measures alongside outcomes of local consultations. The proposed mitigation measures are detailed further in Section 2.4 and have been incorporated into the operational phase scenario.

The PV array tables will be driven into the ground via narrow legs and therefore will not contribute to impermeable areas. Impermeable areas associated with the Development are



therefore limited to the Battery Energy Storage System (BESS), Bull's Lodge Substation, inverters, transformers and an onsite plant and welfare facility. If string inverters are utilised then inverter units will not result in an increase in impermeable areas. As the type of inverter is yet to be confirmed the inverts are assumed to be 'on field' inverters to represent a 'worst case scenario' related to surface water runoff. Sustainable Drainage Strategies (SuDS) for the BESS, onsite plant and welfare facility and Bull's Lodge Substation have been designed by Arcus⁴ and Mott MacDonald⁵ respectively, with such strategies designed to manage any potential increase in surface water runoff rates.

The impermeable areas of the Development are therefore further limited to invert units which have been incorporated into the model and raised above existing topography.

An attenuation pond has been incorporated into the model to the east of Stocks Farm to intercept existing surface water. The proposed pond will not be designed to drain a defined impermeable or infrastructure but to intercept surface water along existing flow routes and thus increase the potential for attenuation of surface water within the Order Limits Boundary.

This has been modelled by depressing a feature of the total area, length and depth of the pond into existing height data. The pond structure has been designed using the Source Control feature within Micro Drainage software to incorporate 1 in 4 slopes. The pond structure will not serve a defined extent of impermeable areas but instead intercept surface water across existing flow routes and has not been designed to a defined rainfall return period. The design details of the pond are as follows:

- Depth 1 m;
- Slope 1 in 4;
- Base area 50 m²; and
- Total area 200.5 m².

To intercept surface water flows leading to Waltham Road and Stocks Farm, a swale has been incorporated to the model to the east of Stocks Farm by depressing a feature of the area, length and depth of the swale into existing height data. The design details of the swale are as follows:

- Length 70 m;
- Depth 1 m;
- Slope 1 in 4;
- Base width 0.5 m²; and
- Total area 6.9 m².

The locations of the proposed pond and swale are shown in Plate 5.

⁴ Arcus, Longfield Solar Farm, SuDS Strategy (2021).

⁵ Motts MacDonald, Drainage Strategy (2021).



Plate 5 – Stocks Farm Pond and Swale Location (Taken from Flood Modeller)

To provide additional surface water attenuation capacity and limit the potential surface water runoff associated with the Development, shallow filter drains along existing flow routes identified within the refined baseline scenario are proposed. The locations of the proposed filter drains are shown in Appendix C.

The filter drains have been incorporated into the model by depressing a feature of the area, length and depth of the drains into existing topography. The lengths of the filter drains vary relative to their location with depths of 1 m and widths of 0.5 m.

3 MITIGATION AND BETTERMENT MEASURES

As mentioned in Section 2.3.3, shallow filter drains are proposed along existing surface water flow routes in accordance with topographic contours. The filter drain features will not incorporate an active outfall but intercept surface water and allow it to infiltrate (as per the baseline scenario) along existing flow routes and provide additional attenuation of surface water throughout the operation of the Development.

The filter drain units will be gravel filled and will be approximately 1 m in depth and 0.5 m in width, with an example filter drain unit shown in Plate 6.



Plate 9: Shallow Filter Drains



Following consultation with surrounding landowners, a pond and swale feature will be implemented to the east of Stocks Farm to intercept and attenuate surface water. This will limit potential surface water flood depths at the surrounding properties and on Waltham Road.

The design parameters of the pond and swale are detailed in Section 2.3.3.

The swale structure will be located at a topographic low point along existing contours in order to intercept surface water without flows dispersing horizontally across the feature and thus leading to overtopping at a low point. An example of a swale structure is shown in Plate 10.

⁶ Malmaynes Solar Farm – Arcus As-built drainage review



Plate 10: Example Perimeter Swale at a Solar Farm Site⁷

4 RESULTS

4.1 Model Stability

To assess model stability the Manning's N Roughness value has been universally increased and decreased by 20 %. Modelling outputs identify a negligible maximal differentiation in surface water depths, thus demonstrating low sensitivity to modelled assumptions. As such, there is confidence that the model is producing credible results.

The maximum total mass error is 8 % with mass error decreasing throughout the iterative modelling process as shown in Table 2.

Simulation	tion Northern Domain Total Mass Error Southern Domain Total Mass Erro	
Baseline	-7 %	8 %
Refined Baseline	-4 %	-4 %
Operational Phase	-4.3 %	-3.7 %

Table 2: 2D Model Total Mass Error

4.2 Baseline Scenario

The 1:100-year (+20 %) modelled outputs for the baseline scenario show maximum surface water flood depths of approximately 0.9 m in isolated area within the Order Limits Boundary, with significant areas of the model extent limited to surface water flood depths of less than 0.1 m.

⁷ Bent Spur Solar Farm - Arcus As-built drainage review



Maximum depths within the Order Limits Boundary are shown at NGR E 576712, N 212576.

Areas within the Order Limits Boundary associated with existing surface water features (i.e. ponds, open land drains) demonstrate depths significantly greater than those upon the surrounding surface due to the depression in topography associated with the features.

The maximum surface water flood depths at the residential properties to the west of the DCO Site are approximately 0.8 m to 1 m. Areas within the vicinity of the Site are shown to have depths exceeding 1 m but are associated with existing surface water bodies.

There are areas on the wider extents of the model active area which are unrepresentative of surface water depths due to water reaching the extent of the model domains and glass-walling (i.e. water cannot exit the model extent).

4.3 Refined Baseline Scenario

To demonstrate the difference in surface water depths during the iterative modelling process, the depths at the maximum point within the Order Limits Boundary and outwith of the Order Limits Boundary for the baseline scenario have been assessed alongside the refined baseline scenario.

There are no significant benefits at the location of the maximal depths during the refined baseline scenario due to the location being a topographic low point, with depths within this areas not impacted by model variances.

The addition of the existing banks adjacent to Stocks Farm into the model is shown to redirect surface water flow routes, which correlates with anecdotal evidence from local landowner consultations.

The baseline and refined baseline scenarios at Stocks Farm are shown in Plates 11 and 12. The insertion of the bank results in additional surface water attenuation within the DCO Site and a reduction in offsite surface water depths.

The 1:100-year (+20 %) surface water depths for the refined baseline scenario are shown in Appendix B.



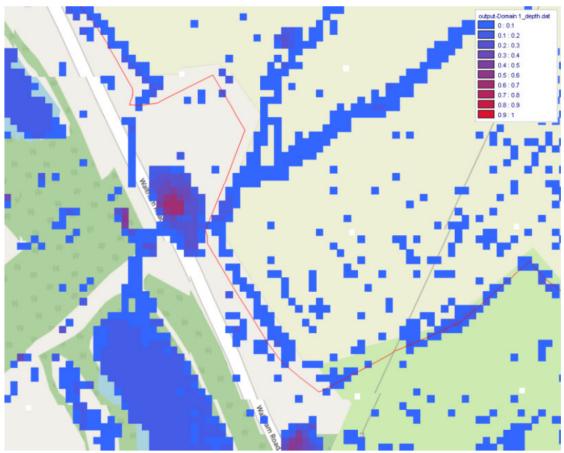
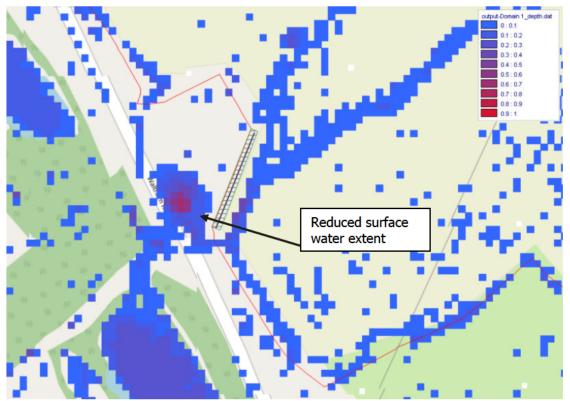


Plate 11: Baseline Model Stocks Farm Output

Plate 12: Refined Baseline Model Stocks Farm Output





4.4 Operational Phase Scenario

To demonstrate the difference in depths during the iterative modelling process the depths at the maximum point within the Order Limits Boundary and outwith of the Order Limits Boundary for the baseline scenario have been assessed alongside the refined baseline scenario.

The location of the maximum surface water depths within the Order Limits Boundary shows no significant reduction. The area with the maximal depths is a topographic low point and is therefore unlikely to have benefited from the benefit measures incorporated within the iterative modelling process.

The filter drains incorporated within the DCO Site are shown to vary in attenuation potential, with the attenuation capacity of the proposed features varying between 50 to 90 %.

Areas to the north west of the Site near Lawn Farm and Noakes House are shown to have benefitted from the insertion of filter drains along identified surface water flow routes as shown in Plate 13 and 14.

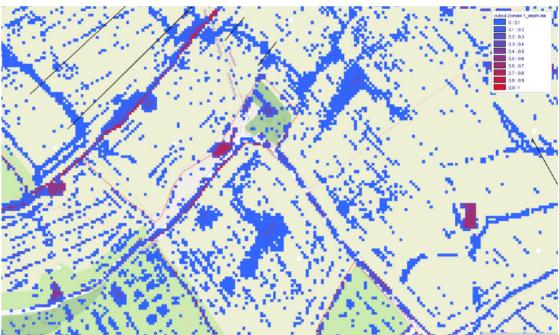
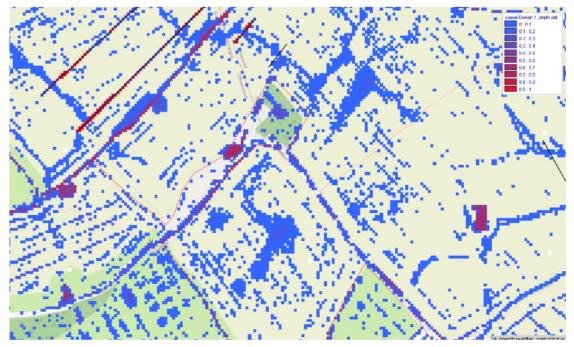


Plate 13: Refined Baseline Scenario Northern Flood Depths

Plate 14: Operational Phase Scenario Northern Flood Depths



The pond and swale units implemented to the east of Stocks Farm are shown to attenuate and intercept surface water along existing flow routes. The maximum depths for the pond and swale during the 1:100-year (+20 %) 2D scenario are shown in Table 3.

Table 3: 2D Model Depth Reductions

Unit	Surface Water Depths
Pond	0.9 m
Swale	0.38 m



Surface water depths at the DCO Site and surrounding areas are shown to reduce at varying locations as detailed in Table 4, demonstrating the benefit of the proposed betterment measures in the interception and attenuation of surface water.

Location	NGR	Baseline Scenario 1:100 (+20%) Depths	Operational Phase Scenario 1:100 (+20%) Depths	Reduction in Depths
Noakes Lane	E 573970, N 213772	0.53 m	0.37 m	0.16 m
Waltham Road	E 573967, N 213 798	0.06 m	0.01 m	0.05 m
Stocks Farm	E 575492, N 212060	0.11 m	0.1 m	0.1 m
Wallace's Lane	E 575822, N 211580	0.82 m	0.7 m	0.12 m
Within OLB	E 576194, N 211921	0.26 m	0.03 m	0.23 m

Table 4: 2D Model Depth Reductions

The 1:100-year (+20%) surface water depths for the operational phase scenario are shown in Appendix C.

5 CONCLUSION

Arcus have produced a 2D hydraulic model utilising Flood Modeller software to demonstrate the surface water flows and depths of the DCO Site and surrounding areas during the 1:100-year+20 % scenario.

To demonstrate the current surface water characteristics of the DCO Site and surrounding areas an iterative process has been applied to enable comparisons of potential betterment measures.

The betterment measures incorporated into the 2D modelling include incremental filter drains, swales and an attenuation pond.

The 2D iterative modelling process demonstrates the benefit of incorporating surface water management measures within the Longfield Solar Farm Development i.e. a reduction in depth and extent of surface water.

The proposed surface water management measures are shown to provide betterment to the surrounding areas and the DCO Site.



APPENDIX A – REFH2 OUTPUT

UK Design Flood Estimation

Generated on 25 August 2021 09:02:37 by reagand Printed from the ReFH2 Flood Modelling software package, version 3.2.7650.24314

Summary of estimate using the Flood Estimation Handbook revitalised flood hydrograph method (ReFH2)

Site details

Checksum: C1D5-8D6A

Site name: FEH_Catchment_Descriptors_578100_211750_REFH2.3

Easting: 578100

Northing: 211750

Country: England, Wales or Northern Ireland

Catchment Area (km²): 75.5

Using plot scale calculations: No

Model: 2.3

Site description: None

Model run: 100 year 1.2 CC

Summary of results

Rainfall - FEH 2013 model (mm):	93.12	Total runoff (ML):	2191.47
Total Rainfall (mm):	87.05	Total flow (ML):	6572.36
Peak Rainfall (mm):	30.85	Peak flow (m ³ /s):	35.52

Parameters

Where the user has overriden a system-generated value, this original value is shown in square brackets after the value used.

* Indicates that the user locked the duration/timestep

Rainfall parameters (Rainfall - FEH 2013 model)

Name	Value	User-defined?
Duration (hh:mm:ss)	18:00:00	No
Timestep (hh:mm:ss)	02:00:00	No
SCF (Seasonal correction factor)	0.99	No
ARF (Areal reduction factor)	0.94	No
Seasonality	Summer [Winter]	Yes
Climate change factor	1.20	Yes

Loss model parameters

Name	Value	User-defined?
Cini (mm)	75.57	No
Cmax (mm)	360.11	No
Use alpha correction factor	No	No
Alpha correction factor	n/a	No

Routing model parameters

Name	Value	User-defined?
Tp (hr)	12.37	No
Up	0.65	No
Uk	0.8	No
Baseflow model parameters		
Name	Value	User-defined?
BF0 (m ³ /s)	1.1	No
BL (hr)	56.33	No
BR	2.02	No
Urbanisation parameters		
Name	Value	User-defined?
Urban area (km²)	1.11	No
Urbext 2000	0.01	No
Impervious runoff factor	0.7	No
Imperviousness factor	0.4	No
Tp scaling factor	0.75	No
Depression storage depth (mm)	0.5	No
Exporting drained area (km ²)	0.00	Yes
Sewer capacity (m³/s)	0.00	Yes

Time series data

Tim (hh:mm:s		Sewer Loss (mm)	Net Rain (mm)	Runoff (m ³ /s)	Baseflow (m³/s)	Total Flow (m³/s)
00:00:0	0 2.572	0.000	0.557	0.000	1.092	1.092
02:00:0	0 4.210	0.000	0.951	0.051	1.056	1.107
04:00:0	0 7.264	0.000	1.757	0.239	1.029	1.268
06:00:0	0 14.053	0.000	3.814	0.674	1.024	1.698
08:00:0	0 30.851	0.000	10.287	1.615	1.066	2.681
10:00:0	0 14.053	0.000	5.559	3.835	1.215	5.050
12:00:0	0 7.264	0.000	3.087	7.485	1.559	9.045
14:00:0	0 4.210	0.000	1.856	11.854	2.167	14.021
16:00:0	0 2.572	0.000	1.158	16.460	3.063	19.523
18:00:0	0.000	0.000	0.000	20.957	4.243	25.200
20:00:0	0.000	0.000	0.000	24.791	5.674	30.465
22:00:0	0.000	0.000	0.000	26.923	7.269	34.192
24:00:0	0.000	0.000	0.000	26.647	8.878	35.525
26:00:0	0.000	0.000	0.000	25.029	10.370	35.399
28:00:0	0.000	0.000	0.000	22.694	11.676	34.370
30:00:0	0.000	0.000	0.000	19.983	12.762	32.745
32:00:0	0.000	0.000	0.000	17.185	13.619	30.803
34:00:0	0.000	0.000	0.000	14.668	14.260	28.928
36:00:0	0.000	0.000	0.000	12.660	14.722	27.382
38:00:0	0.000	0.000	0.000	10.962	15.040	26.002
40:00:0	0.000	0.000	0.000	9.438	15.235	24.674
42:00:0	0.000	0.000	0.000	8.025	15.322	23.347
44:00:0	0.000	0.000	0.000	6.683	15.310	21.993
46:00:0	0.000	0.000	0.000	5.365	15.204	20.569
48:00:0	0.000	0.000	0.000	4.082	15.010	19.092
50:00:0	0.000	0.000	0.000	2.856	14.734	17.589
52:00:0	0.000	0.000	0.000	1.736	14.383	16.119
54:00:0	0.000	0.000	0.000	0.871	13.974	14.845
56:00:0	0.000	0.000	0.000	0.402	13.532	13.934
58:00:0	0.000	0.000	0.000	0.158	13.080	13.238
60:00:0	0.000	0.000	0.000	0.041	12.631	12.672
62:00:0	0.000	0.000	0.000	0.002	12.192	12.193
64:00:0	0.000	0.000	0.000	0.000	11.767	11.767
66:00:0	0.000	0.000	0.000	0.000	11.356	11.356
68:00:0	0.000	0.000	0.000	0.000	10.960	10.960

Page 3 of 6

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Time (hh:mm:ss)	Rain (mm)	Sewer Loss (mm)	Net Rain (mm)	Runoff (m ³ /s)	Baseflow (m³/s)	Total Flow (m³/s)
70:00:00	0.000	0.000	0.000	0.000	10.578	10.578
72:00:00	0.000	0.000	0.000	0.000	10.209	10.209
74:00:00	0.000	0.000	0.000	0.000	9.853	9.853
76:00:00	0.000	0.000	0.000	0.000	9.509	9.509
78:00:00	0.000	0.000	0.000	0.000	9.177	9.177
80:00:00	0.000	0.000	0.000	0.000	8.857	8.857
82:00:00	0.000	0.000	0.000	0.000	8.548	8.548
84:00:00	0.000	0.000	0.000	0.000	8.250	8.250
86:00:00	0.000	0.000	0.000	0.000	7.962	7.962
88:00:00	0.000	0.000	0.000	0.000	7.684	7.684
90:00:00	0.000	0.000	0.000	0.000	7.416	7.416
92:00:00	0.000	0.000	0.000	0.000	7.158	7.158
94:00:00	0.000	0.000	0.000	0.000	6.908	6.908
96:00:00	0.000	0.000	0.000	0.000	6.667	6.667
98:00:00	0.000	0.000	0.000	0.000	6.434	6.434
100:00:00	0.000	0.000	0.000	0.000	6.210	6.210
102:00:00	0.000	0.000	0.000	0.000	5.993	5.993
104:00:00	0.000	0.000	0.000	0.000	5.784	5.784
106:00:00	0.000	0.000	0.000	0.000	5.583	5.583
108:00:00	0.000	0.000	0.000	0.000	5.388	5.388
110:00:00	0.000	0.000	0.000	0.000	5.200	5.200
112:00:00	0.000	0.000	0.000	0.000	5.019	5.019
114:00:00	0.000	0.000	0.000	0.000	4.843	4.843
116:00:00	0.000	0.000	0.000	0.000	4.674	4.674
118:00:00	0.000	0.000	0.000	0.000	4.511	4.511
120:00:00	0.000	0.000	0.000	0.000	4.354	4.354
122:00:00	0.000	0.000	0.000	0.000	4.202	4.202
124:00:00	0.000	0.000	0.000	0.000	4.056	4.056
126:00:00	0.000	0.000	0.000	0.000	3.914	3.914
128:00:00	0.000	0.000	0.000	0.000	3.778	3.778
130:00:00	0.000	0.000	0.000	0.000	3.646	3.646
132:00:00	0.000	0.000	0.000	0.000	3.519	3.519
134:00:00	0.000	0.000	0.000	0.000	3.396	3.396
136:00:00	0.000	0.000	0.000	0.000	3.277	3.277
138:00:00	0.000	0.000	0.000	0.000	3.163	3.163
140:00:00	0.000	0.000	0.000	0.000	3.053	3.053

Printed from the ReFH2 Flood Modelling software package, version 3.2.7650.24314

Time (hh:mm:ss)	Rain (mm)	Sewer Loss (mm)	Net Rain (mm)	Runoff (m ³ /s)	Baseflow (m³/s)	Total Flow (m³/s)
142:00:00	0.000	0.000	0.000	0.000	2.946	2.946
144:00:00	0.000	0.000	0.000	0.000	2.844	2.844
146:00:00	0.000	0.000	0.000	0.000	2.744	2.744
148:00:00	0.000	0.000	0.000	0.000	2.649	2.649
150:00:00	0.000	0.000	0.000	0.000	2.556	2.556
152:00:00	0.000	0.000	0.000	0.000	2.467	2.467
154:00:00	0.000	0.000	0.000	0.000	2.381	2.381
156:00:00	0.000	0.000	0.000	0.000	2.298	2.298
158:00:00	0.000	0.000	0.000	0.000	2.218	2.218
160:00:00	0.000	0.000	0.000	0.000	2.140	2.140
162:00:00	0.000	0.000	0.000	0.000	2.066	2.066
164:00:00	0.000	0.000	0.000	0.000	1.994	1.994
166:00:00	0.000	0.000	0.000	0.000	1.924	1.924
168:00:00	0.000	0.000	0.000	0.000	1.857	1.857
170:00:00	0.000	0.000	0.000	0.000	1.792	1.792
172:00:00	0.000	0.000	0.000	0.000	1.730	1.730
174:00:00	0.000	0.000	0.000	0.000	1.669	1.669
176:00:00	0.000	0.000	0.000	0.000	1.611	1.611
178:00:00	0.000	0.000	0.000	0.000	1.555	1.555
180:00:00	0.000	0.000	0.000	0.000	1.501	1.501
182:00:00	0.000	0.000	0.000	0.000	1.448	1.448
184:00:00	0.000	0.000	0.000	0.000	1.398	1.398
186:00:00	0.000	0.000	0.000	0.000	1.349	1.349
188:00:00	0.000	0.000	0.000	0.000	1.302	1.302
190:00:00	0.000	0.000	0.000	0.000	1.257	1.257
192:00:00	0.000	0.000	0.000	0.000	1.213	1.213
194:00:00	0.000	0.000	0.000	0.000	1.170	1.170
196:00:00	0.000	0.000	0.000	0.000	1.130	1.130

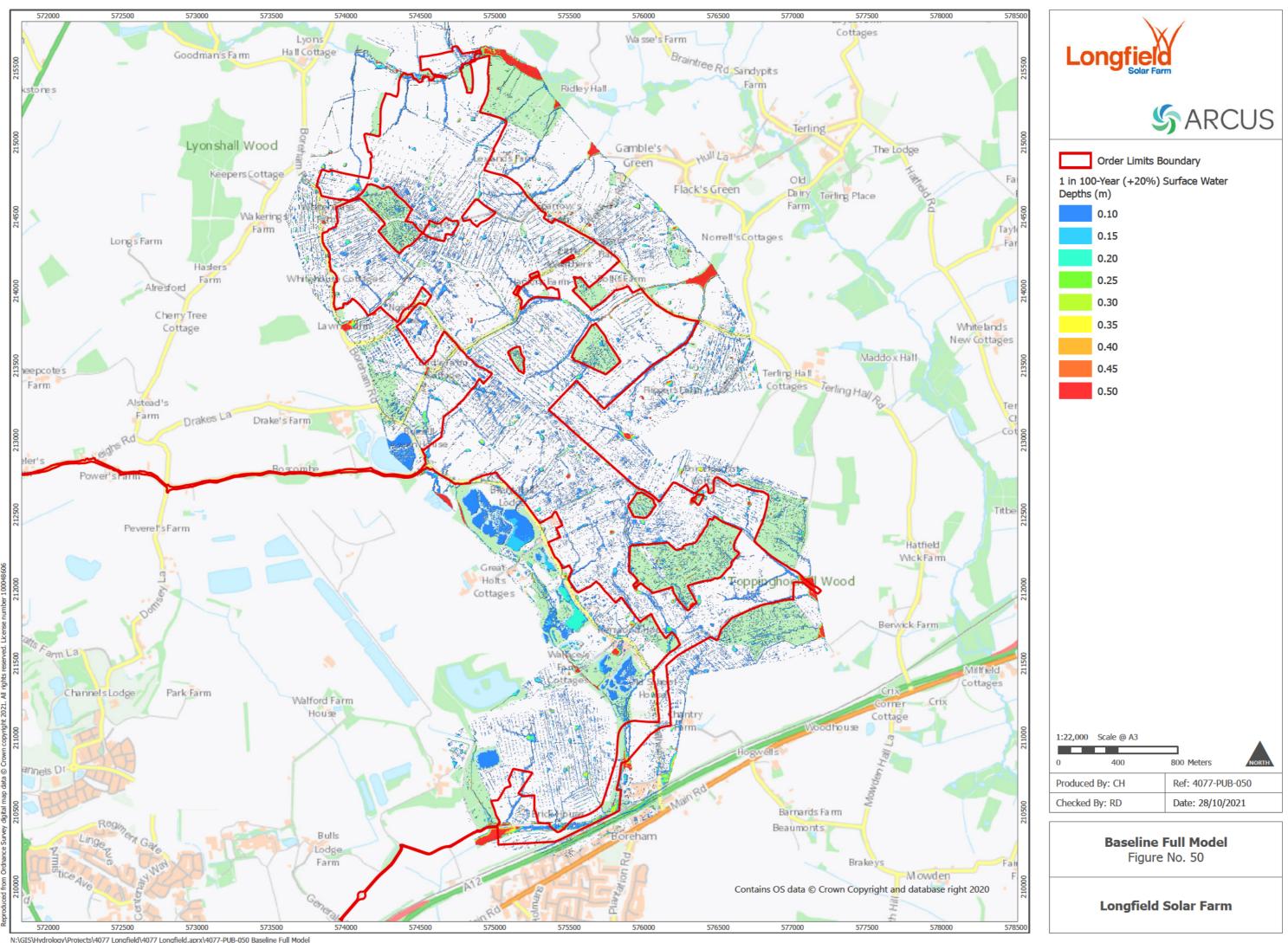
Appendix

Catchment descriptors

Name	Value	User-defined value used?
Area (km²)	75.5	No
ALTBAR	60	No
ASPBAR	151	No
ASPVAR	0.22	No
BFIHOST	0.46	No
BFIHOST19	0.44	No
DPLBAR (km)	12.68	No
DPSBAR (mkm-1)	18.3	No
FARL	0.99	No
LDP	27.82	No
PROPWET (mm)	0.31	No
RMED1H	11.5	No
RMED1D	28.8	No
RMED2D	36.9	No
SAAR (mm)	570	No
SAAR4170 (mm)	592	No
SPRHOST	41.84	No
Urbext2000	0.01	No
Urbext1990	0.01	No
URBCONC	0.73	No
URBLOC	1.22	No
DDF parameter C	-0.02	No
DDF parameter D1	0.27	No
DDF parameter D2	0.27	No
DDF parameter D3	0.25	No
DDF parameter E	0.31	No
DDF parameter F	2.56	No
DDF parameter C (1km grid value)	-0.03	No
DDF parameter D1 (1km grid value)	0.26	No
DDF parameter D2 (1km grid value)	0.29	No
DDF parameter D3 (1km grid value)	0.26	No
DDF parameter E (1km grid value)	0.32	No
DDF parameter F (1km grid value)	2.53	No



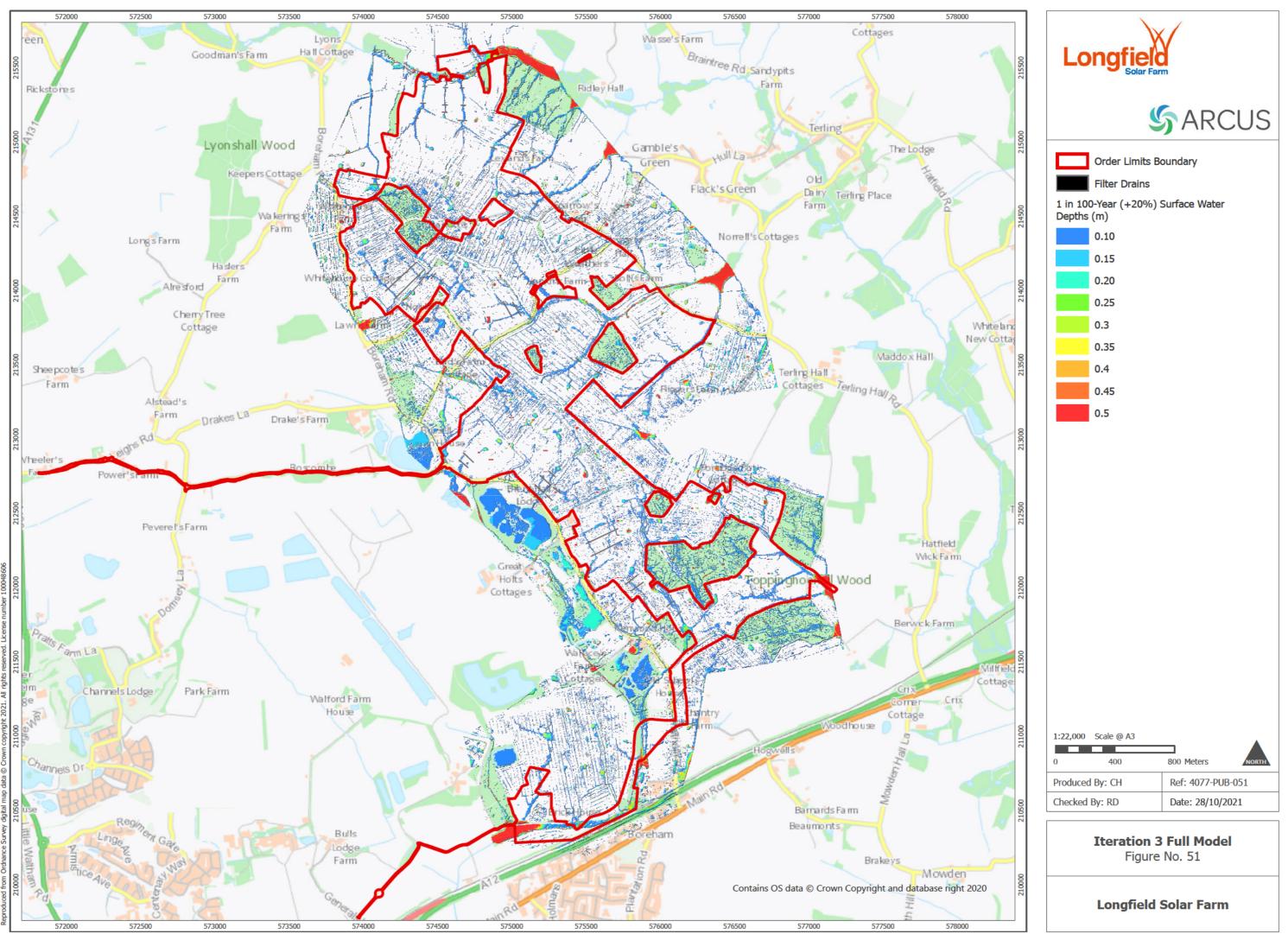
APPENDIX B – REFINED BASELINE SCENARIO 1 IN 100-YEAR +20% SURFACE WATER DEPTHS



N:\GIS\Hydrology\Projects\4077 Longfield\4077 Longfield.aprx\4077-PUB-050 Baseline Full Model



APPENDIX C – OPERATIONAL PHASE SCENARIO 1 IN 100-YEAR +20% SURFACE WATER DEPTHS



N:\GIS\Hydrology\Projects\4077 Longfield\4077 Longfield.aprx\4077-PUB-051 Iteration 3 Full Model